

A phase-type maintenance model considering condition-based inspections and delays before the repairs

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Markov models are widely used in maintenance modelling and system performance analysis due to their computational efficiency and analytical traceability. However, these models are usually restricted by the use of exponential distributions, which are the base of the Markov modelling. Phase-type distributions provide a tool to approximate an adequate distribution, such as Weibull, log-normal and so on, by means of Markov processes.

Our earlier work proposes a phase-type maintenance model considering both condition-based inspections and delays before the repairs, where extra matrices are defined in the modelling of repair delays to keep track of the probability masses to repair. The model provides quite good estimations but is complex and requires good knowledge in its implementation.

This paper aims to get rid of the extra matrices and investigate the modelling of the repair delays with phase-type distributions. An illustration case of road bridges is presented to demonstrate the modelling process and the results.

Keywords: Maintenance modelling, Phase-type distribution, Condition-based inspections, Delays before repairs.

1. Introduction

With the rapid development of monitoring technologies, condition-based maintenance (CBM) is gaining popularity in the past decades. Various maintenance management systems have been developed to store the monitoring results and assist in planning different maintenance tasks. These tasks are often planned based on predefined rules derived from the analysis results of maintenance models, among which Markovian models are used extensively due to their computational efficiency and analytical traceability.

The Markov models in the previous literature are usually restricted by the memory-less property of the Markov Chain, which is shown as exponential

sojourn times between the discrete states. Phase-type (PH) distributions provide a solution to fit general distributions by means of Markov chains and thus capture the non-Markovian deterioration while still taking advantage of the tractability of the Markovian models. The class of PH distributions has strong versatility in approximating a generic distribution, and any non-negative distribution can be approximated arbitrarily close by a PH distribution (Lindqvist and Kjølén, 2018).

In our earlier work, a Markov-based maintenance model is proposed considering both condition-based inspections and deterministic delays before the repairs (see Sun and Vatn (2023)). The deterioration process is approximated with PH distribu-

tions, while the delays before the repairs are modelled by defining extra matrices and keeping track of the probability masses to repair. The model provides quite good estimations but is complex and requires good knowledge in its implementation. In addition, it requires some modification if we want to model the repair delays as a distribution instead of deterministic values.

This paper considered the same case of bridge management in Norway and investigated the modelling of both the bridge deterioration and the repair delays with phase-type distributions. The delays are assumed to be lognormal-distributed instead of deterministic to limit the number of phases required.

The remainder of this paper is organised as follows: Section 2 summarised the background knowledge in phase-type distribution and the fitting approach used in this paper. The detailed modelling process is then presented in Section 3, followed by some numerical results in Section 4. We end with a summary in Section 5. The notations used in this paper are listed in Table 1.

2. Phase-type distributions

Consider a Markov process with state space $\mathcal{S} = \{1, 2, \dots, m, m + 1\}$, where the state $1, 2, \dots, m$ are transient and state $m + 1$ is absorbing. The time to absorption is then said to follow a phase-type distribution. The infinitesimal generator matrix \mathbf{A} is a $(m + 1) \times (m + 1)$ matrix given by:

$$\mathbf{A} = \begin{pmatrix} \mathbf{S} & \mathbf{s} \\ \mathbf{0} & 0 \end{pmatrix} \quad (1)$$

Where \mathbf{S} is a $m \times m$ matrix defining the transition rates among the transient states; \mathbf{s} is a $m \times 1$ vector describing the transition rates from the transient states to the absorbing state; $\mathbf{0}$ is a $1 \times m$ vector of zeros.

An essential task in the implementation of PH distributions is to determine its representation (α, \mathbf{S}) based on either empirical data or probability density functions. Many fitting approaches are available in the literature, generally classified as moment-based and likelihood-based. In this process, the number of phases m and the structure of the transient matrix \mathbf{S} need to be determined.

Table 1. Notations

Notation	Interpretation
α_j	Initial probability vector for a phase-type distribution
\mathbf{S}_j	Transition matrix among the transient states for a phase-type distribution
τ_j	Intervals between inspections for the bridge in state j . It is assumed that $\tau_j = k_j \tau_n$
\mathbb{S}_d	A set of Markov states describing the bridge deterioration
\mathbb{S}_r	A set of Markov states describing the repair process
β_u^j	Initial probability for the u th phase of \mathbb{S}_r for vector \mathbf{P}^j
μ_u^j	Transition rate from the u th phase of \mathbb{S}_r for vector \mathbf{P}^j
$P_i^{j,l_j}(t)$	Probability of the bridge being in macro state i at time t while following an inspection regime $\{j, l_j\}$
\mathbf{A}^j	Full transition matrix for \mathbf{P}^{j,l_j} vectors
\mathbf{A}_d	Transition matrix among \mathbb{S}_d
$\mathbf{A}_{r,u}^j$	Transition matrix from the u th phase to the $u + 1$ th phase of \mathbb{S}_r in \mathbf{A}^j
\mathbf{B}^j	Full inspection matrix for \mathbf{P}^{j,l_j} vectors
\mathbf{B}_u^j	Transition rates from macro state j to the u th phase of \mathbb{S}_r in \mathbf{B}^j
e_S, e_M, e_L	Probability that a minor repair, a major repair or rehabilitation successfully restores the bridge to the desired state
ϕ^j	Intended state change before and after the repair for the bridge in state j
θ	A maintenance action (inspection or a type of repair)
Θ	A set of all possible actions
C_θ	Cost per action θ
d_f	Expected duration of being in state 5
C_f	Penalty per unit time when the bridge is in state 5

Estimating all transition probabilities can be computationally expensive for a full PH distribution. Therefore, some special cases are usually adopted to limit the model complexity. A widely used subclass is the Acyclic PH (APH) distribution, where the \mathbf{S} matrix becomes an upper triangular matrix, can always be represented in three canonical forms, and the parameter estimation essentially reduces to PH fitting with these canonical

forms (Okamura and Dohi, 2016). This paper uses the moment-based fitting function in the Butools toolbox (BuTools, 2015) to derive the parameters for the PH distribution.

2.1. PH expansion of multi-state Markov models

A multi-state Markov model can be extended to its PH expansion by approximating the sojourn times at each state with PH distributions and merging them together. In the merging, the transition rates to the absorbing state, i.e. the next macro state, are split according to the initial probability vector of the next states to ensure a good fitting (see proof in Laskowska and Vatn (2020)). Suppose there is a Markov chain with n macro states, where the sojourn times at state $j \in \{1, 2, \dots, n - 1\}$ are approximated with PH distributions of m_j phases. Fig.1 visualizes the concept of merging.

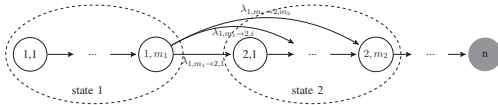


Fig. 1. PH expansion of multi-state Markov model

The macro state $j + 1$ can be regarded as the absorbing state from the intermediate state j, m_j , where the transition rates can be derived by

$$\begin{aligned} & [\lambda_{j,m_j \rightarrow \{j+1\},1}, \dots, \lambda_{j,m_j \rightarrow \{j+1\},m_{j+1}}] \\ & = \alpha_{j+1} \cdot \lambda_{j \rightarrow j+1} \end{aligned} \quad (2)$$

The expansion of the multi-state Markov model can be represented with initial probability vector $P_0 = [\alpha_1, 0]$ and transition matrix A .

$$\begin{aligned} A &= \begin{pmatrix} S_1 & S_{1 \rightarrow 2} & & & \\ & S_2 & S_{2 \rightarrow 3} & & \\ & & \ddots & & \\ & & & S_{n-1} & S_{n-1 \rightarrow n} \\ & & & & 0 \end{pmatrix}, \quad (3) \\ S_{j \rightarrow j+1} &= \begin{pmatrix} 0 \\ \vdots \\ \alpha_{j+1} \cdot \lambda_{j,m_j \rightarrow j+1} \end{pmatrix} \end{aligned}$$

According to the Chapman-Kolmogorov differential equation, the probability distribution at time t

can be derived by:

$$P(t) = P_0 \cdot e^{At} \quad (4)$$

3. Model Description

3.1. Model assumptions

The following assumptions are adopted in this paper.

- (i) The bridge deterioration is described by five macro discrete condition states. The sojourn times for bridge deterioration are assumed Weibull-distributed. Based on the parameters found in Fang and Sun (2019), three states are required for approximating the sojourn times in state 1 and state 2, while two states are enough for state 3 and state 4.
- (ii) The bridge condition can be revealed by condition-based inspections and after each repair. The time for the next inspection is based on the bridge condition revealed at the current inspection. Let t_0 be the current inspection time, and j be the bridge condition revealed from an inspection; the time for the next inspection would be at $t_0 + \tau_j$, where $j \in \{1, 2, 3, 4, 5\}$ denotes the bridge state found during the inspection. It is assumed that $\tau_j = k_j \tau_5$, where k_j are all integers for simplification.
- (iii) All inspections are perfect and can reveal the actual bridge condition.
- (iv) There are significant waiting times before the conduction of repairs. The waiting times depend on the bridge's condition revealed during an inspection and are assumed to be lognormal-distributed, which is a distribution commonly used for modelling repair times (Rausand et al., 2021). Extra penalties will be triggered when the bridge is in state 5.
- (v) There are three levels of repairs: minor repair (MiRep), major repair (MaRep) and rehabilitation (Rehab). The repairs will most restore the bridge to the desired state but may fail to achieve the planned bridge improvement in some cases. Table 2 shows the ideal bridge improvement and success probability of different levels of repairs.

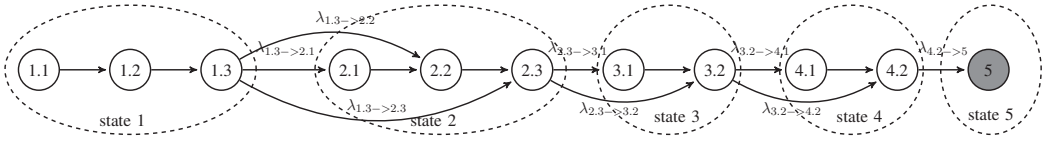


Fig. 3. Markov process for P^j when there is no repair planned for bridge in state j

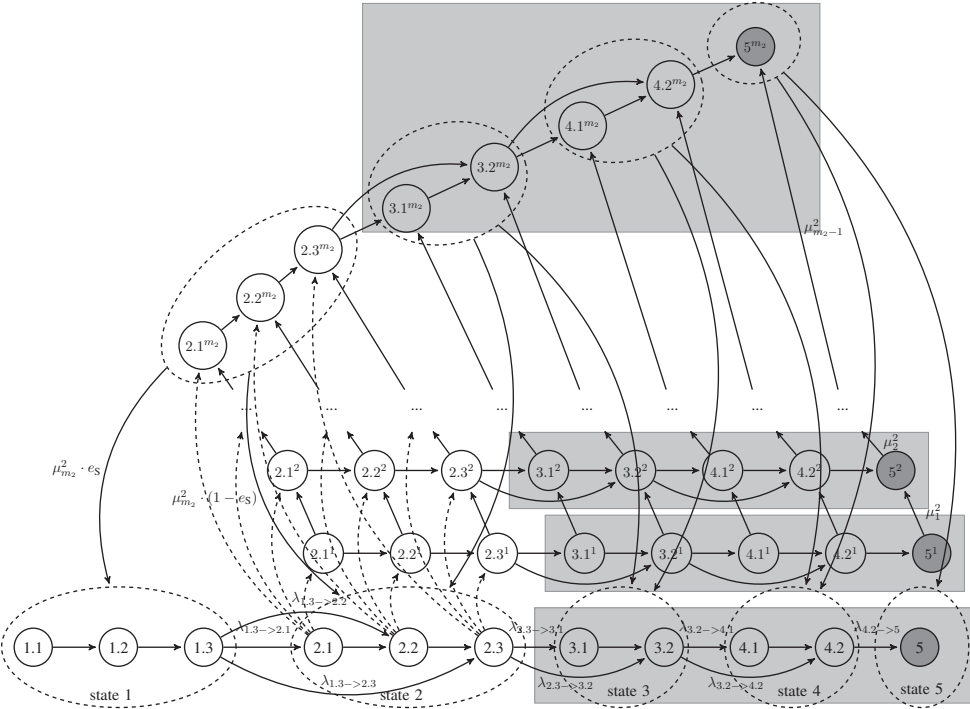


Fig. 4. Markov process for P^2 when minor repairs will be planned for the bridge in state 2

strategy for the bridge in macro state j .

$$A_d = \begin{pmatrix} S_1 & S_{1 \rightarrow 2} & & & & \\ & S_2 & S_{2 \rightarrow 3} & & & \\ & & S_3 & S_{3 \rightarrow 4} & & \\ & & & S_4 & S_{4 \rightarrow 5} & \\ & & & & & 0 \end{pmatrix} \quad (6)$$

$$A_{r,u}^j = \text{diag}(\mu_u^j, \dots, \mu_u^j) \quad (7)$$

During the inspections, the probability mass in S_d is moved to S_r , described in Fig. 4 and Fig. 5 as the dashed lines. This process can be realized by multiplying the P^j vector with its inspection matrices B^j . For instance, Eq. 8 gives the inspection matrix for P^2 . Here B_u^2 is in the dimension

of state 2, I_1 , I_{3-5} and I_r denotes the identity matrix in the dimension of macro state 1, macro state 3 to 5 and the states in S_r respectively. Meanwhile, the shuffling of the probability mass described in Fig. 2 will be done, representing a change of inspection regime. It should be noted that here P_2^j includes the probability mass in state 2.1, 2.2, 2.3, 2.1¹, 2.2¹, 2.3¹, ..., 2.1^{m_r}, 2.2^{m_r}, 2.3^{m_r}; P_3^j includes the probability mass in state 3.1, 3.2, ..., 3.1^{m_r}, 3.2^{m_r} (see the states marked with the grey box). The same applies to the mod-

the whole vector at each inspection.

$$E(N_{\text{Insp}}) = \sum_{t=0}^T \sum_{i=\tau_{\{j,l_j\}}} \sum_{i \in \mathbb{S}_d \cup \mathbb{S}_r} P_i^{j,l_j}(t) \quad (11)$$

4. Numerical Results

4.1. Input parameters

Table 3 presents the bridge deterioration and repair delays parameters. The parameters for bridge deterioration are estimated from a study by Fang and Sun (2019) based on the bridge inspection data in Shanghai. The parameters for repair delays are based on the maintenance requirement in Norway, which can be summarised as follows: For bridges in state 1, no maintenance action is required; for bridges in state 2, maintenance should be conducted between four to ten years; for bridges in state 3, maintenance should be conducted between one to three years; for bridges in state 4 and 5, maintenance should be conducted within six months. Table 4 presents the unit costs for different maintenance actions.

Table 3. Input parameters

State	Deterioration parameters		Repair parameters	
	Scale (yr)	Shape	Mean (yr)	Sigma
1	27.531	1.458	/	/
2	26.025	1.599	4.421	0.142
3	31.788	1.328	3.167	0.149
4	21.266	1.217	1.375	0.149
5	/	/	1.375	0.149

4.2. Bridge performance and the number of actions

To verify the proposed model, we compared The result with a Monte Carlo Simulation (MCS), where the sojourn time at each main state follows a Weibull distribution and the times before repairs are lognormal-distributed with the parameters in Table 3.

Consider a maintenance strategy with $\tau_1 = 14$ years, $\tau_2 = 6$ years, $\tau_3 = 2$ years, $\tau_4 = 3$ months and all repairs intend to restore the bridge to state 1, the time-dependent state probabilities from both approaches are presented in Fig. 6, and

Table 4. Cost values

	C'_{Insp}	C'_{MiRep}	C'_{MaRep}	C'_{Rehab}
Cost (10 ³ NOK)	500	1,000	2,000	4,000

the expected number of inspections and repairs are presented in Table 5. As we can see, the proposed approach gives very close results with the MCS regarding the bridge performance and the expected number of actions.

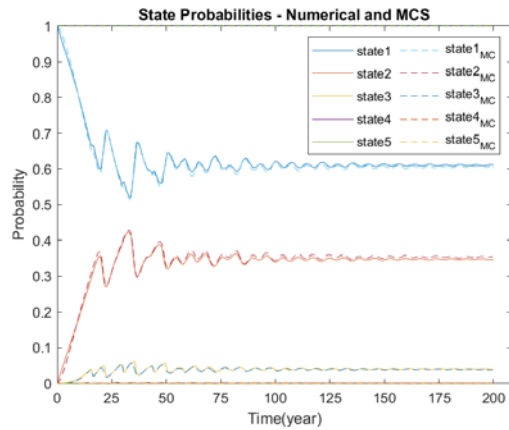


Fig. 6. Time-dependent state probabilities for the illustrative strategy

Table 5. Expected number of actions and system performance

	Proposed Model	MCS
$E(N_{\text{Insp}})$	15.891	15.838
$E(N_{\text{MiRep}})$	3.727	3.689
$E(N_{\text{MaRep}})$	1.184	1.226
$E(N_{\text{Rehab}})$	0.081	0.086
d_f (yr)	0.00091	0.00084

4.3. Optimisation result

Different strategies can be evaluated with the proposed approach to search for an optimal solution. In practice, the inspections are usually planned over a time of years. The reference interval τ_5 is set to be one month, and only integer years are considered for τ_1, τ_2 and τ_3 . For τ_1 , the upper

limit for searching is set to be the expected sojourn time while the ones for τ_2 and τ_3 are set to be its expected waiting times before repair. The repair for bridges in state 4 must be done within half a year. Therefore, τ_4 is between 1 month and half a year. Considering a large number of potential solutions, the Genetic Algorithm toolbox in MATLAB is used to search for an optimal solution, with a simulated time of 200 years and a stopping condition of 30 stall generations.

Table 6 presents the optimal solution considering different penalties when the bridge is in state 5. Its expected number of actions, the duration in state 5 and the expected annual costs are evaluated with the MCS of 2,000,000 replications. As we can see, with a higher penalty, the model leads to strategies with more frequent inspections, more early repairs and shorter duration in state 5, which is consistent with our analysis.

Table 6. Optimisation results considering different C_f

C_f (10^3 NOK/yr)	20,000	50,000
Inspection Intervals	$\tau_1 = 12$ years, $\tau_2 = 7$ years, $\tau_3 = 2$ years, $\tau_4 = 6$ months	$\tau_1 = 10$ years, $\tau_2 = 5$ years, $\tau_3 = 2$ years, $\tau_4 = 6$ months
Repair Strategy	$\mu_2 = \text{MiRep}, \mu_3 = \text{MaRep}, \mu_4 = \mu_5 = \text{Rehab}$	
$E(N_{\text{Insp}})$	16.116	21.48
$E(N_{\text{MiRep}})$	4.269	4.261
$E(N_{\text{MaRep}})$	1.032	1.066
$E(N_{\text{Rehab}})$	0.004	0.004
d_f (yr)	6.193×10^{-4}	2.252×10^{-4}

5. Summary

This paper presents an extension of our earlier work on the PH model considering condition-based inspections and significant delays before the repairs (Sun and Vatn, 2023). In contrast to the deterministic delay times modelled with extra matrices, this paper considered lognormal-distributed delay times and investigated modelling such delays with PH distributions. Monte Carlo Simulation is used to verify the results.

With this model, the expected system performance can be assessed given the inspection intervals, the

delays before the repairs and the repair actions at different system conditions. An illustration case of road bridges is presented to demonstrate the modelling approach and its potential use in maintenance optimisation.

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